BS EN 1992-1-1:2004

Incorporating corrigendum January 2008 and November 2010

# Eurocode 2: Design of concrete structures —

Part 1-1: General rules and rules for buildings

 $ICS\ 91.010.30;\ 91.080.40$ 



# National foreword

This British Standard is the UK implementation of EN 1992-1-1:2004, incorporating corrigendum January 2008 and November 2010. It supersedes DD ENV 1992-1-1:1992, DD ENV 1992-1-3:1996, DD ENV 1992-1-4:1996, DD ENV 1992-1-5:1996, DD ENV 1992-1-6:1996 which are withdrawn.

The start and finish of text introduced or altered by corrigendum is indicated in the text by tags. For example, text altered by CEN corrigendum January 2008 is indicated in the text by  $\boxed{\mbox{AC}_1\mbox{}}$   $\boxed{\mbox{AC}_1\mbox{}}$  and CEN corrigendum November 2010 by  $\boxed{\mbox{AC}_2\mbox{}}$ .

The structural Eurocodes are divided into packages by grouping Eurocodes for each of the main materials, concrete, steel, composite concrete and steel, timber, masonry and aluminium, this is to enable a common date of withdrawal (DOW) for all the relevant parts that are needed for a particular design. The conflicting national standards will be withdrawn at the end of the coexistence period, after all the EN Eurocodes of a package are available.

Following publication of the EN, there is a period of 2 years allowed for the national calibration period during which the National Annex is issued, followed by a three year coexistence period. During the coexistence period Member States will be encouraged to adapt their national provisions to withdraw conflicting national rules before the end of the coexistent period. The Commission in consultation with Member States is expected to agree the end of the coexistence period for each package of Eurocodes.

At the end of this coexistence period, the national standard(s) will be withdrawn.

In the UK, the corresponding national standards are:

- BS 8110-1:1997, Structural use of concrete Part 1: Code of practice for design and construction;
- BS 8110-2:1985, Structural use of concrete Part 2: Code of practice for special circumstances;
- BS 8110-3:1985, Structural use of concrete Part 3: Design charts for singly reinforced beams, doubly reinforced beams and rectangular columns:

and based on this transition period, these standards will be withdrawn on a date to be announced.

The UK participation in its preparation was entrusted by Technical Committee B/525, Building and civil engineering structures, to Subcommittee B/525/2, Structural use of concrete.

This British Standard was published under the authority of the Standards Policy and Strategy Committee on 23 December 2004

© BSI 2011

### Amendments/corrigenda issued since publication

Date	Comments	
30 June 2008	Implementation of CEN corrigendum January 2008	
31 August 2011	Implementation of CEN corrigendum November 2010	

A list of organizations represented on this subcommittee can be obtained on request to its secretary.

Where a normative part of this EN allows for a choice to be made at the national level, the range and possible choice will be given in the normative text, and a note will qualify it as a Nationally Determined Parameter (NDP). NDPs can be a specific value for a factor, a specific level or class, a particular method or a particular application rule if several are proposed in the EN.

To enable EN 1992-1-1 to be used in the UK, the NDPs are published in a National Annex, which is available from BSI.

This publication does not purport to include all the necessary provisions of a contract. Users are responsible for its correct application.

Compliance with a British Standard cannot confer immunity from legal obligations.

# EUROPEAN STANDARD NORME EUROPÉENNE

**EUROPÄISCHE NORM** 

EN 1992-1-1

December 2004

ICS 91.010.30: 91.080.40

Incorporating corrigenda January 2008 and November 2010 Supersedes ENV 1992-1-1:1991, ENV 1992-1-3:1994, ENV 1992-1-4:1994, ENV 1992-1-5:1994, ENV 1992-1-6:1994, ENV 1992-3:1998

#### **English version**

# Eurocode 2: Design of concrete structures - Part 1-1: General rules and rules for buildings

Eurocode 2: Calcul des structures en béton - Partie 1-1 : Règles générales et règles pour les bâtiments Eurocode 2: Bemessung und konstruktion von Stahlbetonund Spannbetontragwerken - Teil 1-1: Allgemeine Bemessungsregeln und Regeln für den Hochbau

This European Standard was approved by CEN on 16 April 2004.

CEN members are bound to comply with the CEN/CENELEC Internal Regulations which stipulate the conditions for giving this European Standard the status of a national standard without any alteration. Up-to-date lists and bibliographical references concerning such national standards may be obtained on application to the Central Secretariat or to any CEN member.

This European Standard exists in three official versions (English, French, German). A version in any other language made by translation under the responsibility of a CEN member into its own language and notified to the Central Secretariat has the same status as the official versions.

CEN members are the national standards bodies of Austria, Belgium, Cyprus, Czech Republic, Denmark, Estonia, Finland, France, Germany, Greece, Hungary, Iceland, Ireland, Italy, Latvia, Lithuania, Luxembourg, Malta, Netherlands, Norway, Poland, Portugal, Slovakia, Slovenia, Spain, Sweden, Switzerland and United Kingdom.



EUROPEAN COMMITTEE FOR STANDARDIZATION COMITÉ EUROPÉEN DE NORMALISATION EUROPÄISCHES KOMITEE FÜR NORMUNG

Management Centre: Avenue Marnix 17, B-1000 Brussels

#### **Contents List**

_	
4	General
	(-Andrai

- 1.1 Scope
  - 1.1.1 Scope of Eurocode 2
  - 1.1.2 Scope of Part 1-1 of Eurocode 2
- 1.2 Normative references
  - 1.2.1 General reference standards
  - 1.2.2 Other reference standards
- 1.3 Assumptions
- 1.4 Distinction between principles and application rules
- 1.5 Definitions
  - 1.5.1 General
  - 1.5.2 Additional terms and definitions used in this Standard
    - 1.5.2.1 Precast structures
    - 1.5.2.2 Plain or lightly reinforced concrete members
    - 1.5.2.3 Unbonded and external tendons
    - 1.5.2.4 Prestress
- 1.6 Symbols
- 2. Basis of design
- 2.1 Requirements
  - 2.1.1 Basic requirements
  - 2.1.2 Reliability management
  - 2.1.3 Design working life, durability and quality management
- 2.2 Principles of limit state design
- 2.3 Basic variables
  - 2.3.1 Actions and environment influences
    - 2.3.1.1 General
    - 2.3.1.2 Thermal effects
    - 2.3.1.3 Differential settlements/movements
    - 2.3.1.4 Prestress
  - 2.3.2 Material and product properties
    - 2.3.2.1 General
    - 2.3.2.2 Shrinkage and creep
  - 2.3.3 Deformations of concrete
  - 2.3.4 Geometric data
    - 2.3.4.1 General
    - 2.3.4.2 Supplementary requirements for cast in place piles
- 2.4 Verification by the partial factor method
  - 2.4.1 General
  - 2.4.2 Design values
    - 2.4.2.1 Partial factor for shrinkage action
    - 2.4.2.2 Partial factors for prestress
    - 2.4.2.3 Partial factor for fatigue loads
    - 2.4.2.4 Partial factors for materials
    - 2.4.2.5 Partial factors for materials for foundations
  - 2.4.3 Combinations of actions
  - 2.4.4 Verification of static equilibrium EQU
- 2.5 Design assisted by testing
- 2.6 Supplementary requirements for foundations
- 2.7 Requirements for fastenings

- 3. Materials
- 3.1 Concrete
  - 3.1.1 General
  - 3.1.2 Strength
  - 3.1.3 Elastic deformation
  - 3.1.4 Creep and shrinkage
  - 3.1.5 Stress-strain relation for non-linear structural analysis
  - 3.1.6 Design compressive and tensile strengths
  - 3.1.7 Stress-strain relations for the design of sections
  - 3.1.8 Flexural tensile strength
  - 3.1.9 Confined concrete
- 3.2 Reinforcing steel
  - 3.2.1 General
  - 3.2.2 Properties
  - 3.2.3 Strength
  - 3.2.4 Ductility characteristics
  - 3.2.5 Welding
  - 3.2.6 Fatigue
  - 3.2.7 Design assumptions
- 3.3 Prestressing steel
  - 3.3.1 General
  - 3.3.2 Properties
  - 3.3.3 Strength
  - 3.3.4 Ductility characteristics
  - 3.3.5 Fatique
  - 3.3.6 Design assumptions
  - 3.3.7 Prestressing tendons in sheaths
- 3.4 Prestressing devices
  - 3.4.1 Anchorages and couplers
    - 3.4.1.1 General
    - 3.4.1.2 Mechanical properties
      - 3.4.1.2.1 Anchored tendons
      - 3.4.1.2.2 Anchored devices and anchorage zones
  - 3.4.2 External non-bonded tendons
    - 3.4.2.1 General
    - 3.4.2.2 Anchorages
- 4. Durability and cover to reinforcement
- 4.1 General
- 4.2 Environmental conditions
- 4.3 Requirements for durability
- 4.4 Methods of verifications
  - 4.4.1 Concrete cover
    - 4.4.1.1 General
    - 4.4.1.2 Minimum cover,  $c_{min}$
    - 4.4.1.3 Allowance in design for tolerance
- 5. Structural analysis
- 5.1 General
  - 5.1.1 General requirements
  - 5.1.2 Special requirements for foundations
  - 5.1.3 Load cases and combinations
  - 5.1.4 Second order effects

- 5.2 Geometric imperfections
- 5.3 Idealisation of the structure
  - 5.3.1 Structural models for overall analysis
  - 5.3.2 Geometric data
    - 5.3.2.1 Effective width of flanges (all limit states)
    - 5.3.2.2 Effective span of beams and slabs in buildings
- 5.4 Linear elastic analysis
- 5.5 Linear analysis with limited redistribution
- 5.6 Plastic analysis
  - 5.6.1 General
  - 5.6.2 Plastic analysis for beams, frames and slabs
  - 5.6.3 Rotation capacity
  - 5.6.4 Analysis with struts and tie models
- 5.7 Non-linear analysis
- 5.8 Analysis of second order effects with axial load
  - 5.8.1 Definitions
  - 5.8.2 General
  - 5.8.3 Simplified criteria for second order effects
    - 5.8.3.1 Slenderness Criterion for isolated members
    - 5.8.3.2 Slenderness and effective length of isolated members
    - 5.8.3.3 Global second order effects in buildings
  - 5.8.4 Creep
  - 5.8.5 Methods of analysis
  - 5.8.6 General method
  - 5.8.7 Method based on nominal stiffness
    - 5.8.7.1 General
    - 5.8.7.2 Nominal stiffness
    - 5.8.7.3 Moment magnification factor
  - 5.8.8 Method based on nominal curvature
    - 5.8.8.1 General
    - 5.8.8.2 Bending moments
    - 5.8.8.3 Curvature
  - 5.8.9 Biaxial bending
- 5.9 Lateral instability of slender beams
- 5.10 Prestressed members and structures
  - 5.10.1 General
  - 5.10.2 Prestressing force during tensioning
    - 5.10.2.1 Maximum stressing force
    - 5.10.2.2 Limitation of concrete stress
    - 5.10.2.3 Measurements
  - 5.10.3 Prestress force
  - 5.10.4 Immediate losses of prestress for pre-tensioning
  - 5.10.5 Immediate losses of prestress for post-tensioning
    - 5.10.5.1 Losses due to the instantaneous deformation of concrete
    - 5.10.5.2 Losses due to friction
    - 5.10.5.3 Losses at anchorage
  - 5.10.6 Time dependent losses of prestress for pre- and post-tensioning
  - 5.10.7 Consideration of prestress in analysis
  - 5.10.8 Effects of prestressing at ultimate limit state
  - 5.10.9 Effects of prestressing at serviceability limit state and limit state of fatigue
- 5.11 Analysis for some particular structural members

- 6. Ultimate limit states (ULS)
- 6.1 Bending with or without axial force
- 6.2 Shear
  - 6.2.1 General verification procedure
  - 6.2.2 Members not requiring design shear reinforcement
  - 6.2.3 Members requiring design shear reinforcement
  - 6.2.4 Shear between web and flanges
  - 6.2.5 Shear at the interface between concretes cast at different times
- 6.3 Torsion
  - 6.3.1 General
  - 6.3.2 Design procedure
  - 6.3.3 Warping torsion
- 6.4 Punching
  - 6.4.1 General
  - 6.4.2 Load distribution and basic control perimeter
  - 6.4.3 Punching shear calculation
  - 6.4.4 Punching shear resistance of slabs and column bases without shear reinforcement
  - 6.4.5 Punching shear resistance of slabs and column bases with shear reinforcement
- 6.5 Design with strut and tie models
  - 6.5.1 General
  - 6.5.2 Struts
  - 6.5.3 Ties
  - 6.5.4 Nodes
- 6.6 Anchorages and laps
- 6.7 Partially loaded areas
- 6.8 Fatigue
  - 6.8.1 Verification conditions
  - 6.8.2 Internal forces and stresses for fatigue verification
  - 6.8.3 Combination of actions
  - 6.8.4 Verification procedure for reinforcing and prestressing steel
  - 6.8.5 Verification using damage equivalent stress range
  - 6.8.6 Other verifications
  - 6.8.7 Verification of concrete under compression or shear
- 7. Serviceability limit states (SLS)
- 7.1 General
- 7.2 Stress limitation
- 7.3 Crack control
  - 7.3.1 General considerations
  - 7.3.2 Minimum reinforcement areas
  - 7.3.3 Control of cracking without direct calculation
  - 7.3.4 Calculation of crack widths
- 7.4 Deflection control
  - 7.4.1 General considerations
  - 7.4.2 Cases where calculations may be omitted
  - 7.4.3 Checking deflections by calculation
- 8 Detailing of reinforcement and prestressing tendons General
- 8.1 General
- 8.2 Spacing of bars
- 8.3 Permissible mandrel diameters for bent bars
- 8.4 Anchorage of longitudinal reinforcement
  - 8.4.1 General

- 8.4.2 Ultimate bond stress
- 8.4.3 Basic anchorage length
- 8.4.4 Design anchorage length
- 8.5 Anchorage of links and shear reinforcement
- 8.6 Anchorage by welded bars
- 8.7 Laps and mechanical couplers
  - 8.7.1 General
  - 8.7.2 Laps
  - 8.7.3 Lap length
  - 8.7.4 Transverse reinforcement in the lap zone
    - 8.7.4.1 Transverse reinforcement for bars in tension
    - 8.7.4.2 Transverse reinforcement for bars permanently in compression
  - 8.7.5 Laps for welded mesh fabrics made of ribbed wires
    - 8.7.5.1 Laps of the main reinforcement
    - 8.7.5.2 Laps of secondary or distribution reinforcement
- 8.8 Additional rules for large diameter bars
- 8.9 Bundled bars
  - 8.9.1 General
  - 8.9.2 Anchorage of bundles of bars
  - 8.9.3 Lapping bundles of bars
- 8.10 Prestressing tendons
  - 8.10.1 Arrangement of prestressing tendons and ducts
    - 8.10.1.1 General
    - 8.10.1.2 Pre-tensioned tendons
    - 8.10.1.3 Post-tension ducts
  - 8.10.2 Anchorage of pre-tensioned tendons
    - 8.10.2.1 General
    - 8.10.2.2 Transfer of prestress
    - 8.10.2.3 Anchorage of tendons for the ultimate limit state
  - 8.10.3 Anchorage zones of post-tensioned members
  - 8.10.4 Anchorages and couplers for prestressing tendons
  - 8.10.5 Deviators
- 9. Detailing of members and particular rules
- 9.1 General
- 9.2 Beams
  - 9.2.1 Longitudinal reinforcement
    - 9.2.1.1 Minimum and maximum reinforcement areas
    - 9.2.1.2 Other detailing arrangements
    - 9.2.1.3 Curtailment of the longitudinal tension reinforcement
    - 9.2.1.4 Anchorage of bottom reinforcement at an end support
    - 9.2.1.5 Anchorage of bottom reinforcement at intermediate supports
  - 9.2.2 Shear reinforcement
  - 9.2.3 Torsion reinforcement
  - 9.2.4 Surface reinforcement
  - 9.2.5 Indirect supports
- 9.3 Solid slabs
  - 9.3.1 Flexural reinforcement
    - 9.3.1.1 General
    - 9.3.1.2 Reinforcement in slabs near supports
    - 9.3.1.3 Corner reinforcement
    - 9.3.1.4 Reinforcement at the free edges

9.3.2 Shear reinforcement 9.4 Flat slabs 9.4.1 Slab at internal columns 9.4.2 Slab at edge columns 9.4.3 Punching shear reinforcement 9.5 Columns 9.5.1 General 9.5.2 Longitudinal reinforcement 9.5.3 Transverse reinforcement 9.6 Walls 9.6.1 General 9.6.2 Vertical reinforcement 9.6.3 Horizontal reinforcement 9.6.4 Transverse reinforcement 9.7 Deep beams 9.8 **Foundations** 9.8.1 Pile caps 9.8.2 Column and wall footings 9.8.2.1 General 9.8.2.2 Anchorage of bars 9.8.3 Tie beams 9.8.4 Column footing on rock 9.8.5 Bored piles Regions with discontinuity in geometry or action 9.9 Tying systems 9.10 9.10.1 General 9.10.2 Proportioning of ties 9.10.2.1 General 9.10.2.2 Peripheral ties 9.10.2.3 Internal ties 9.10.2.4 Horizontal ties to columns and/or walls 9.10.2.5 Vertical ties 9.10.3 Continuity and anchorage of ties Additional rules for precast concrete elements and structures 10. 10.1 General 10.1.1 Special terms used in this section Basis of design, fundamental requirements 10.2 10.3 Materials 10.3.1 Concrete 10.3.1.1 Strength 10.3.1.2 Creep and shrinkage 10.3.1 Prestressing steel 10.3.2.1 Technological properties of prestressing steel Structural analysis 10.5 10.5.1 General 10.5.2 Losses of prestress Particular rules for design and detailing 10.9.1 Restraining moments in slabs 10.9.2 Wall to floor connections 10.9.3 Floor systems

10.9.4 Connections and supports for precast elements

- 10.9.4.1 Materials
- 10.9.4.2 General rules for design and detailing of connections
- 10.9.4.3 Connections transmitting compressive forces
- 10.9.4.4 Connections transmitting shear forces
- 10.9.4.5 Connections transmitting bending moments or tensile forces
- 10.9.4.6 Half joints
- 10.9.4.7 Anchorage of reinforcement at supports
- 10.9.5 Bearings
  - 10.9.5.1 General
  - 10.9.5.2 Bearings for connected (non-isolated) members
  - 10.9.5.3 Bearings for isolated members
- 10.9.6 Pocket foundations
  - 10.9.6.1 General
  - 10.9.6.2 Pockets with keyed surfaces
  - 10.9.6.3 Pockets with smooth surfaces
- 10.9.7 Tying systems
- 11. Lightweight aggregated concrete structures
- 11.1 General
  - 11.1.1 Scope
  - 11.1.2 Special symbols
- 11.2 Basis of design
- 11.3 Materials
  - 11.3.1 Concrete
  - 11.3.2 Elastic deformation
  - 11.3.3 Creep and shrinkage
  - 11.3.4 Stress-strain relations for structural analysis
  - 11.3.5 Design compressive and tensile strengths
  - 11.3.6 Stress-strain relations for the design of sections
  - 11.3.7 Confined concrete
- 11.4 Durability and cover to reinforcement
  - 11.4.1 Environmental conditions
  - 11.4.2 Concrete cover and properties of concrete
- 11.5 Structural analysis
  - 11.5.1 Rotational capacity
- 11.6 Ultimate limit states
  - 11.6.1 Members not requiring design shear reinforcement
  - 11.6.2 Members requiring design shear reinforcement
  - 11.6.3 Torsion
    - 11.6.3.1 Design procedure
  - 11.6.4 Punching
    - 11.6.4.1 Punching shear resistance of slabs and column bases without shear reinforcement
    - 11.6.4.2 Punching shear resistance of slabs and column bases with shear reinforcement
  - 11.6.5 Partially loaded areas
  - 11.6.6 Fatigue
- 11.7 Serviceability limit states
- 11.8 Detailing of reinforcement General
  - 11.8.1 Permissible mandrel diameters for bent bars
  - 11.8.2 Ultimate bond stress
- 11.9 Detailing of members and particular rules

- 11.10 Additional rules for precast concrete elements and structures
- 11.12 Plain and lightly reinforced concrete structures
- 12. Plain and lightly reinforced concrete structures
- 12.1 General
- 12.2 Basis of design
  - 12.2.1 Strength
- 12.3 Materials
  - 12.3.1 Concrete: additional design assumptions
- 12.5 Structural analysis: ultimate Limit states
- 12.6 Ultimate limit states
  - 12.6.1 Design resistance to bending and axial force
  - 12.6.2 Local Failure
  - 12.6.3 Shear
  - 12.6.4 Torsion
  - 12.6.5 Ultimate limit states induced by structural deformation (buckling)
    - 12.6.5.1 Slenderness of columns and walls
    - 12.6.5.2 Simplified design method for walls and columns
- 12.7 Serviceability limit states
- 12.9 Detailing of members and particular rules
  - 12.9.1 Structural members
  - 12.9.2 Construction joints
  - 12.9.3 Strip and pad footings

#### **Annexes**

Α	(Informative)	Modification of partial factors for materials
В	(Informative)	Creep and shrinkage strain
С	(Normative)	Reinforcement properties
D	(Informative)	Detailed calculation method for prestressing steel relaxation losses
Ε	(Informative)	Indicative Strength Classes for durability
F	(Informative)	Reinforcement expressions for in-plane stress conditions
G	(Informative)	Soil structure interaction
Н	(Informative)	Global second order effects in structures
1	(Informative)	Analysis of flat slabs and shear walls
J	(Informative)	Examples of regions with discontinuity in geometry or action

#### **Foreword**

This European Standard EN 1992, Eurocode 2: Design of concrete structures: General rules and rules for buildings, has been prepared by Technical Committee CEN/TC250 « Structural Eurocodes », the Secretariat of which is held by BSI. CEN/TC250 is responsible for all Structural Eurocodes.

This European Standard shall be given the status of a National Standard, either by publication of an identical text or by endorsement, at the latest by June 2005, and conflicting National Standards shall be withdrawn at latest by March 2010.

This Eurocode supersedes ENV 1992-1-1, 1992-1-3, 1992-1-4, 1992-1-5, 1992-1-6 and 1992-3.

According to the CEN-CENELEC Internal Regulations, the National Standard Organisations of the following countries are bound to implement these European Standard: Austria, Belgium, Cyprus, Czech Republic, Denmark, Estonia, Finland, France, Germany, Greece, Hungary,

Iceland, Ireland, Italy, Latvia, Lithuania, Luxembourg, Malta, Netherlands, Norway, Poland, Portugal, Slovakia, Slovenia, Spain, Sweden, Switzerland and United Kingdom.

## **Background to the Eurocode programme**

In 1975, the Commission of the European Community decided on an action programme in the field of construction, based on article 95 of the Treaty. The objective of the programme was the elimination of technical obstacles to trade and the harmonisation of technical specifications.

Within this action programme, the Commission took the initiative to establish a set of harmonised technical rules for the design of construction works which, in a first stage, would serve as an alternative to the national rules in force in the Member States and, ultimately, would replace them.

For fifteen years, the Commission, with the help of a Steering Committee with Representatives of Member States, conducted the development of the Eurocodes programme, which led to the first generation of European codes in the 1980s.

In 1989, the Commission and the Member States of the EU and EFTA decided, on the basis of an agreement<sup>1</sup> between the Commission and CEN, to transfer the preparation and the publication of the Eurocodes to CEN through a series of Mandates, in order to provide them with a future status of European Standard (EN). This links *de facto* the Eurocodes with the provisions of all the Council's Directives and/or Commission's Decisions dealing with European standards (*e.g.* the Council Directive 89/106/EEC on construction products - CPD - and Council Directives 93/37/EEC, 92/50/EEC and 89/440/EEC on public works and services and equivalent EFTA Directives initiated in pursuit of setting up the internal market).

The Structural Eurocode programme comprises the following standards generally consisting of a number of Parts:

EN 1990	Eurocode 0:	Basis of Structural Design
EN 1991	Eurocode 1:	Actions on structures
EN 1992	Eurocode 2:	Design of concrete structures
EN 1993	Eurocode 3:	Design of steel structures
EN 1994	Eurocode 4:	Design of composite steel and concrete structures
EN 1995	Eurocode 5:	Design of timber structures
EN 1996	Eurocode 6:	Design of masonry structures
EN 1997	Eurocode 7:	Geotechnical design
EN 1998	Eurocode 8:	Design of structures for earthquake resistance
EN 1999	Eurocode 9:	Design of aluminium structures

Eurocode standards recognise the responsibility of regulatory authorities in each Member State and have safeguarded their right to determine values related to regulatory safety matters at national level where these continue to vary from State to State.

#### Status and field of application of eurocodes

The Member States of the EU and EFTA recognise that Eurocodes serve as reference documents for the following purposes :

- as a means to prove compliance of building and civil engineering works with the essential

Agreement between the Commission of the European Communities and the European Committee for Standardisation (CEN) concerning the work on EUROCODES for the design of building and civil engineering works (BC/CEN/03/89).